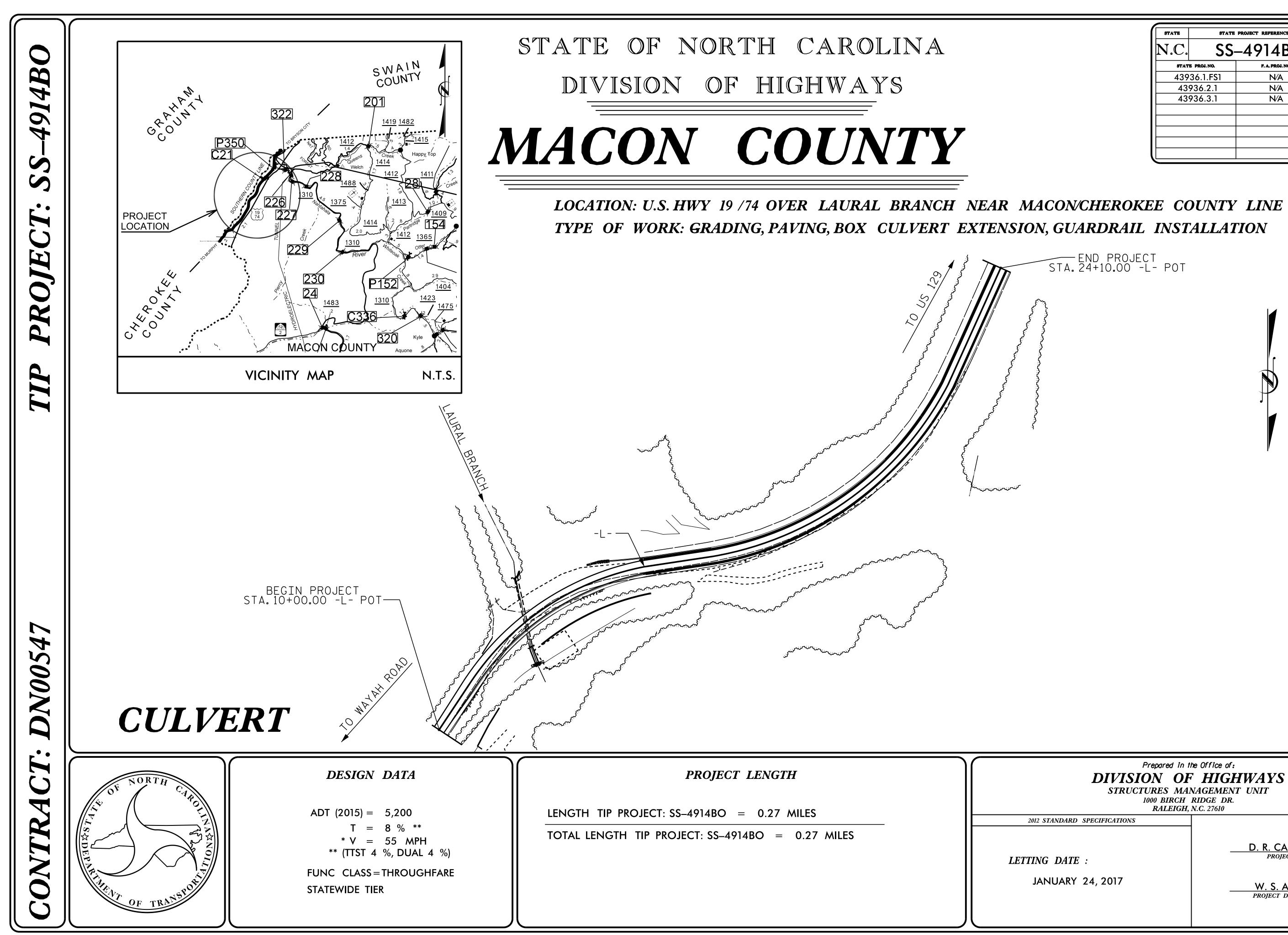
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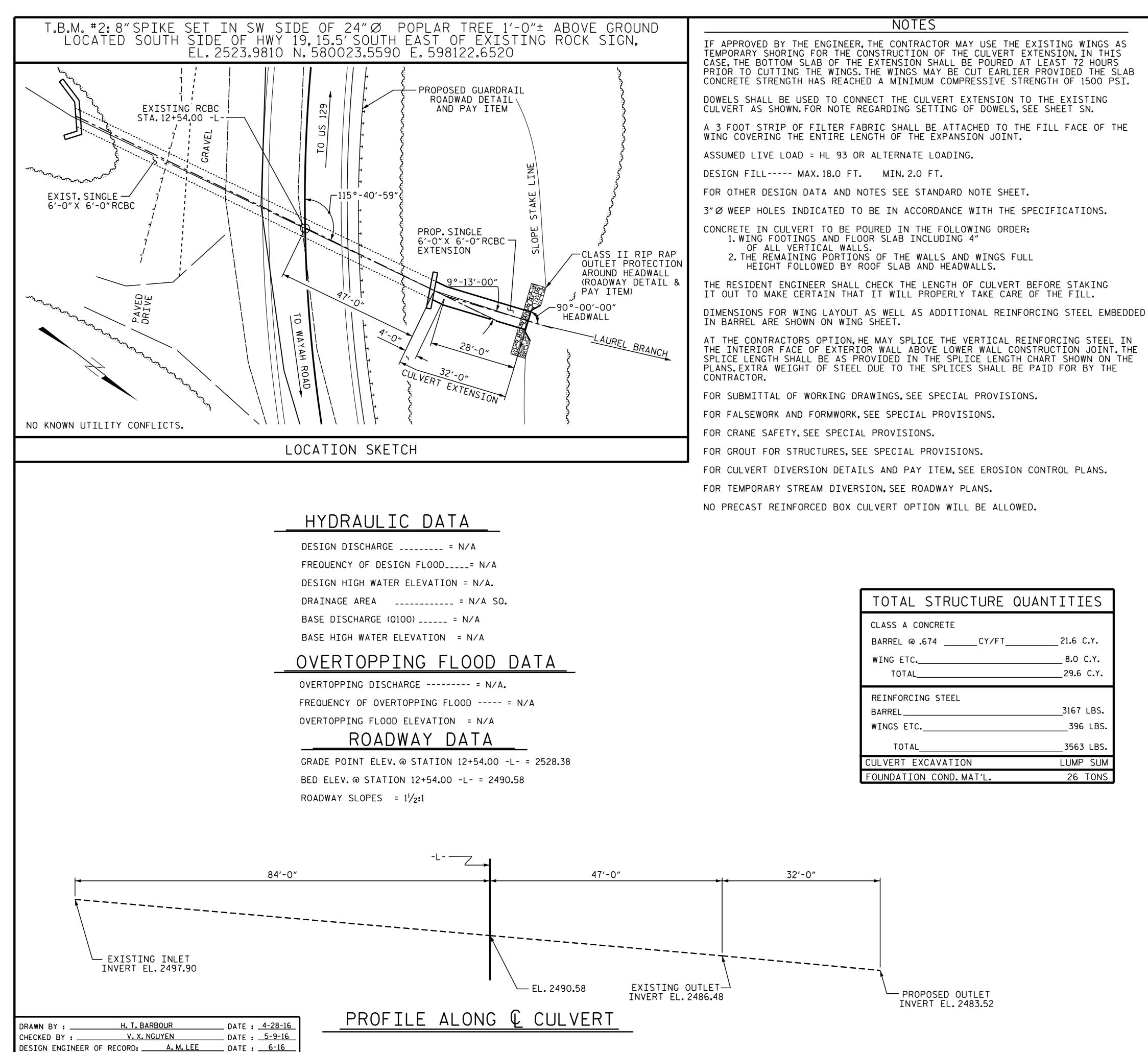
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STATE	STATE STATE PROJECT REFERENCE NO. SHEET NO. TOTAL SHEETS					
N.C.	N.C. SS-4914BO					
STAT	STATE PROJ. NO. F. A. PROJ. NO.		DESCRIP	TION		
439	36.1.FS1	N⁄A	P.E			
43	936.2.1	N⁄A	R∕W, U	TIL.		
43	936.3.1	N⁄A	CONS	ST.		
]		

END PROJECT STA. 24+10.00 -L- POT

Prepared in th DIVISION OF STRUCTURES MAN 1000 BIRCH RALEIGH,	HIGHWAYS NAGEMENT UNIT RIDGE DR.
DATE : JARY 24, 2017	D. R. CALHOUN, P.E. PROJECT ENGINEER W. S. ARAFAT, P.E.
	PROJECT DESIGN ENGINEER



20-JUN-2016 10:13 S:\DPG3\Wael\Culver+\D14_SD_CU_01.dgn warafat

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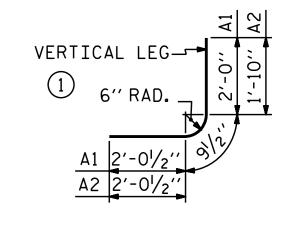
TOTAL STRUCTURE QUAN	TITIES
CLASS A CONCRETE	
BARREL @ .674CY/FT	21.6 C.Y.
WING ETC	8.0 C.Y.
TOTAL	29.6 C.Y.
REINFORCING STEEL	
BARREL	3167 LBS.
WINGS ETC	396 LBS.
TOTAL	3563 LBS.
CULVERT EXCAVATION	LUMP SUM
FOUNDATION COND. MAT'L.	26 TONS

REI	NFOR	CING	BAR	SCHE)ULE
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	85	#4	1	4'-10"	274
A2	85	# 5	1	4'-8"	414
A100	64	#4	STR.	6'-11"	296
A101	1	#4	STR.	5′-5″	4
A102	1	#4	STR.	3'-7"	2
A103	1	#4	STR.	1'-9"	1
A200	64	# 5	STR.	6'-11"	462
A201	1	# 5	STR.	5′-5″	6
A202	1	# 5	STR.	3'-7"	4
A203	1	# 5	STR.	1'-9"	2
B1	63	#4	STR.	6'-10"	288
B2	85	#4	STR.	5'-4"	303
C1	76	#4	STR.	17'-0"	863
D1	20	#6	STR.	2'-6"	75
F1	9	#4	STR.	7'-0"	42
G1	2	# 5	STR.	7'-0"	15
S1	6	#8	STR.	7'-3"	116
				71	

REINFORCING STEEL

3167 LBS.

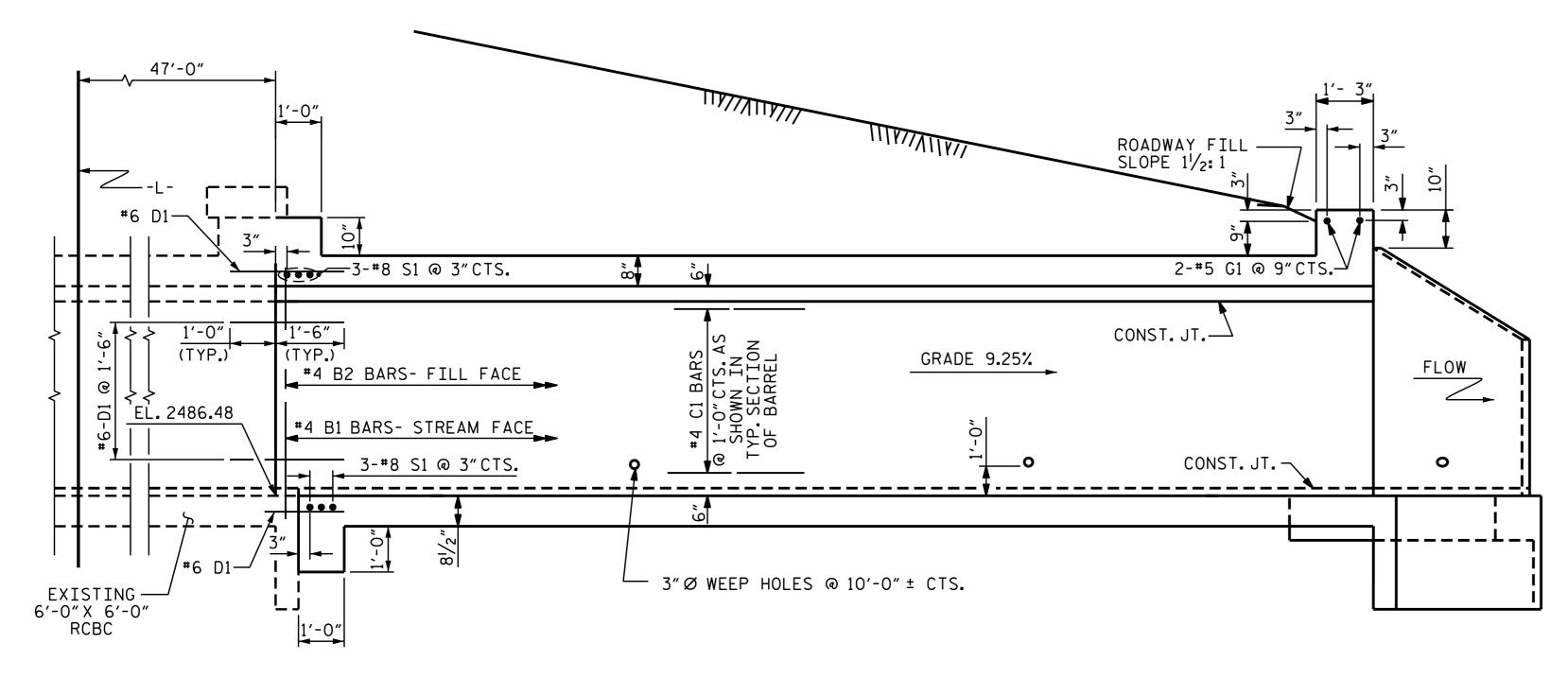
BAR TYPE



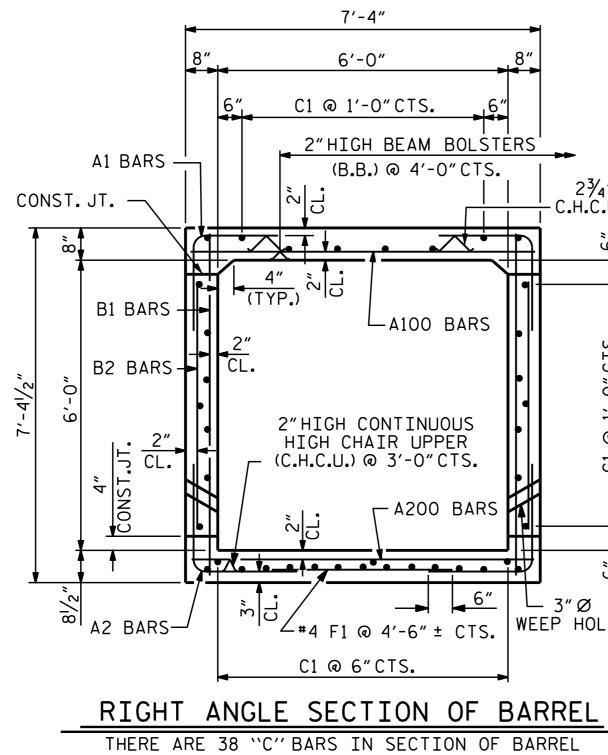
BAR DIMENSIONS	ARE	OUT	ΤO	OUT	
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SPLICE CHART				
BAR	SIZE	SPLICE LENGTH		
B1	#4	1'-5"		
C1	#4	1'-11"		

SEAL 14855 14875 14875	M	no. <u>SS-</u> ACON <u>12+54</u> .	CO	<u>30</u> unty L-
6/20/2016 6/20/2016 6/20/2016 FTH CARO FESSION SEAL 17230 NGNEER ITZ30 NGNEER NO NGNEER NO NGNEER NO NGNEER NO NGNEER NO NGNEER NGNE	SINGL CONCRE 90°	STATE OF NORTH CAR ENT OF TRAN RALEIGH E 6 F T TE BOX -00'-00 HT EXTE	NSPORTA X 6 CUL X SKE	FT. VERT W
4139C12A32AB406 6/20/2016		REVISIONS		SHEET NO.
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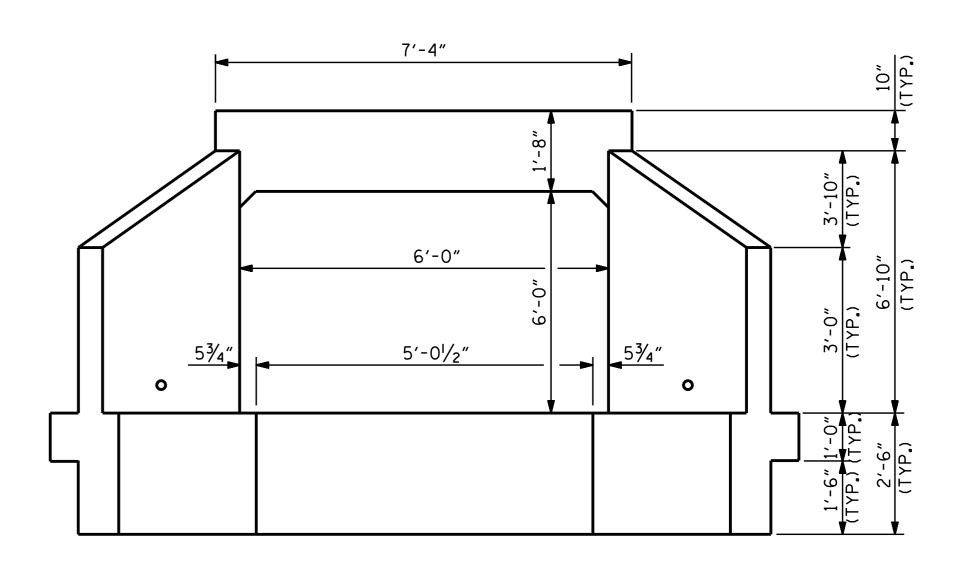
CULVERT SECTION NORMAL TO ROADWAY



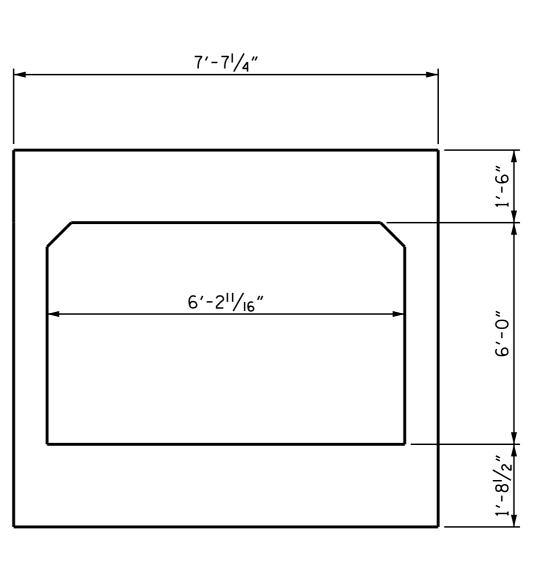
G.R.P. C.R.K.			
BY BY			
E.L.R. CHECKED A.R.B. CHECKED			
8-28-92 BY 8-22-89 BY V 8-22-1989			
REVISED REVISED REDRAWN	ASSEMBLED BY : <u>H.T.BARBOUR</u> CHECKED BY : <u>V.X.NGUYEN</u>	DATE : <u>4-27-16</u> DATE : <u>5-9-16</u>	SPECIAL
R RI	DRAWN BY :R.WRIGHT CHECKED BY :A.R.BISSETTE	DATE : <u>AUG.1989</u> DATE : <u>AUG.1989</u>	STANDARD

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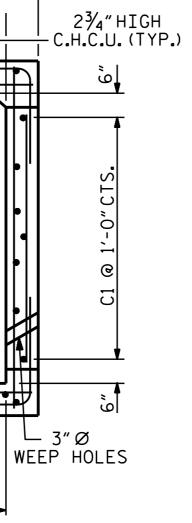
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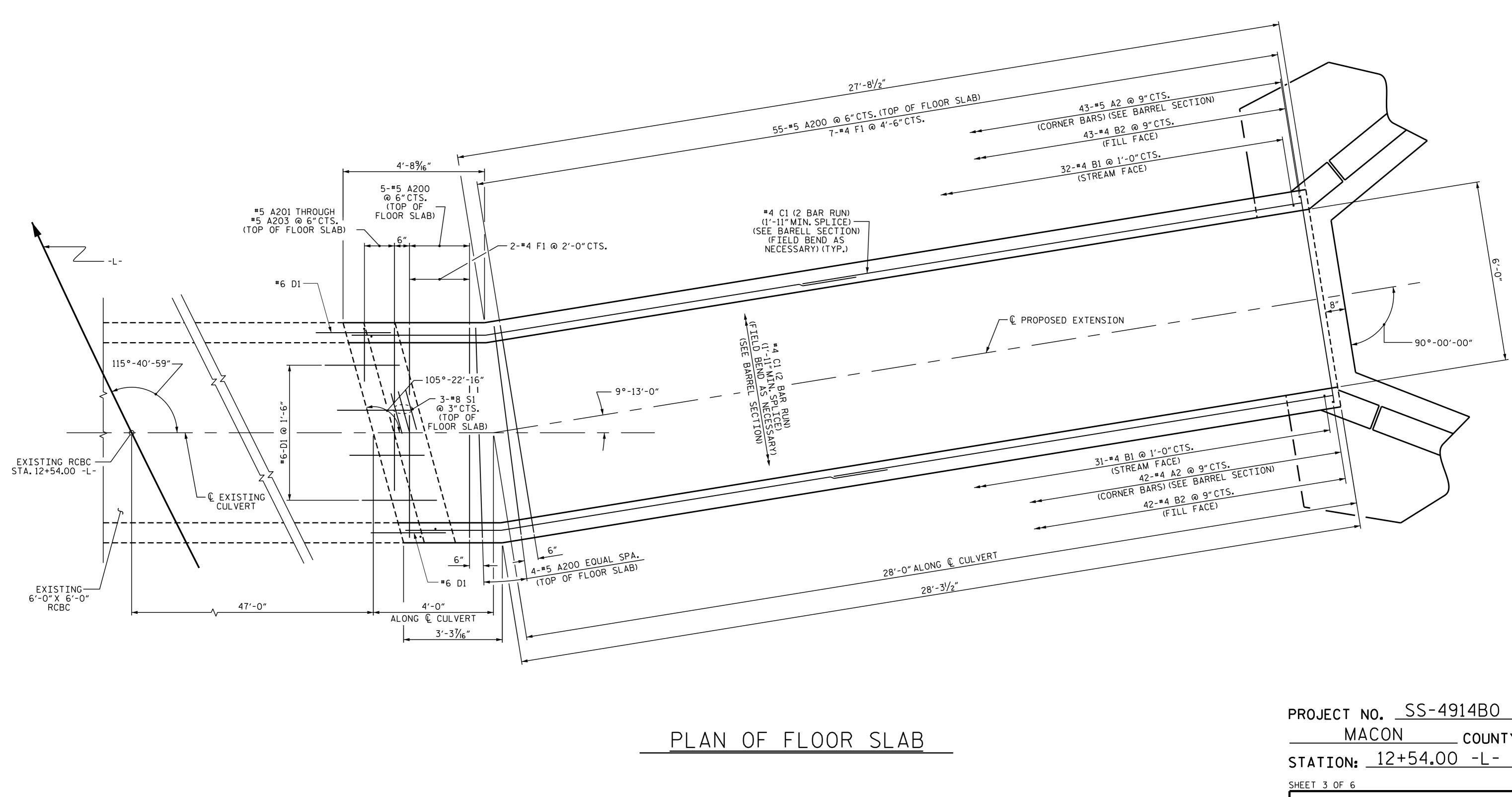
OUTLET END ELEVATION



INLET END



ELEVATION	PROJECT	NO.	SS-	4914	30
		MACO	N	CO	UNTY
	STATION: 12+54.00 -L-				
	SHEET 2 OF	6			
TH CAROLINE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH				
SEAL 17230 S. ARAF MUNICIPALITY	CONCF	RETE	BOX	X6 CUL	VERT
DocuSigned by: Wael Orafat 4139C12A32AB406				"ŠKĒ NSIC	
6/20/2016		REVISIO	NS		SHEET NO.
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STD. NO. CB11					

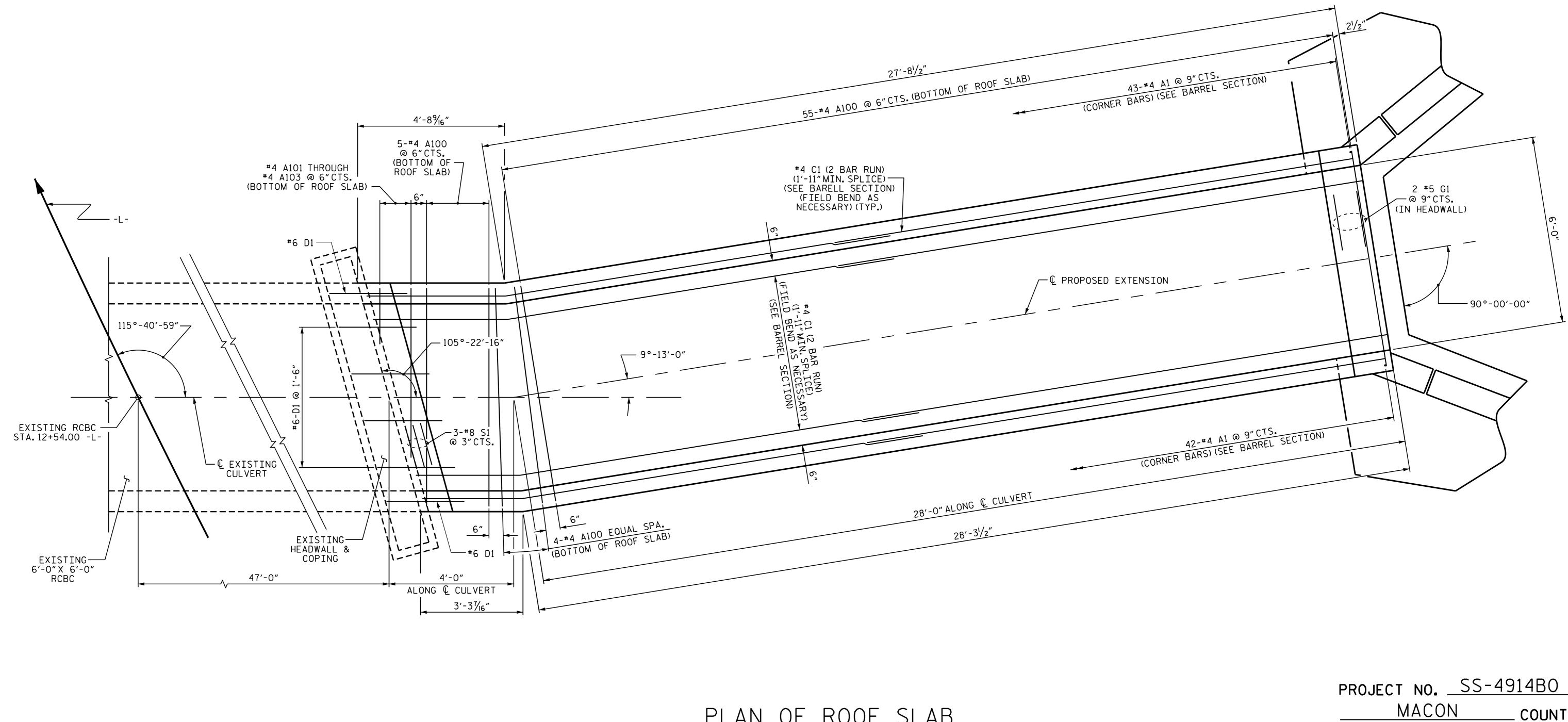


DRAWN BY :	н. т. ва	RBOUR	DATE :	4-27-16
CHECKED BY :	V. X. N	IGUYEN	DATE :	5-9-15
DESIGN ENGINEER	OF RECORD:	A. M. LEE	DATE :	

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PROJECT NO. <u>SS-4914</u> <u>MACON</u> CO STATION: <u>12+54.00</u> SHEET 3 OF 6	BO DUNTY ·L-
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTA RALEIGH SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SEAL 17230 SINGLE 6 FT. X 6 CONCRETE BOX CUL 90°-00'-00" SKE (RIGHT EXTENSION	FT. VERT W
6/20/2016 REVISIONS	SHEET NO.
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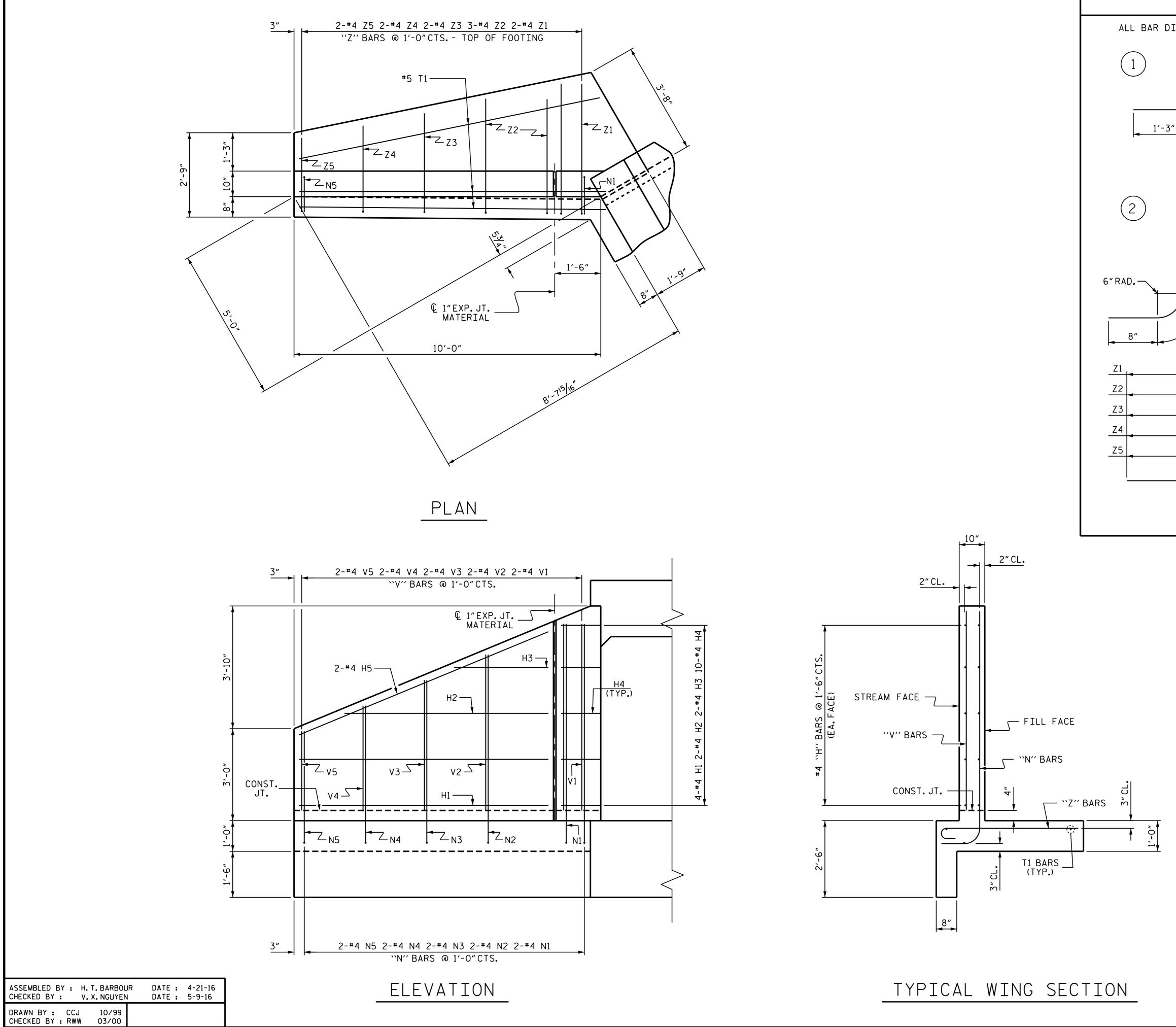
DRAWN BY :	H. T. BARBOUR	DATE : <u>4-27-16</u>
CHECKED BY :	V. X. NGUYEN	DATE : <u>5-9-16</u>
DESIGN ENGINEER	OF RECORD: A. M. LE	
		20- 1110-2016 10-23

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<u>plan of roof slab</u>

	PROJECT NO. <u>SS</u> MACON	COUNTY
	STATION: <u>12+5</u> 4	1.00 -L-
	SHEET 4 OF 6	
TH CAROLINE	STATE OF NORTH OF DEPARTMENT OF TR RALEIGH	
OR SEAL IT230	SINGLE 6 FI CONCRETE BO 90°-00'-0	X CULVERT
DocuSigned by: Wael Orafat 4139C12A32AB406	(RĨGHT ĔXĬ	
6/20/2016	REVISIONS	SHEET NO.
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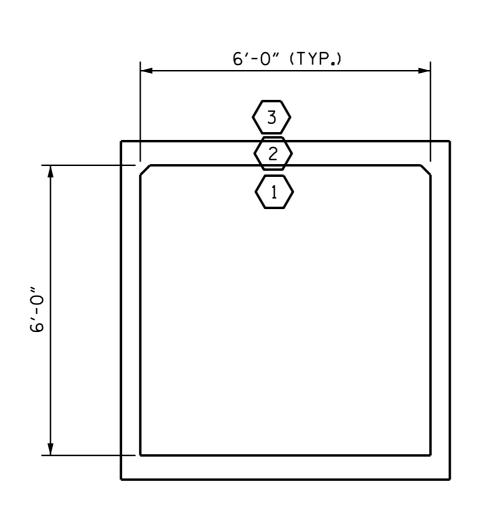
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BAR TYPES	BILL OF MATERIAL				L	
DIMENSIONS ARE OUT TO OUT.	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
JIMENSIONS ARE OUT TO OUT.	H1	8	#4	STR	8'-1"	43
	H2	4	#4	STR	6′-8″	18
2'-0'	Н3	4	#4	STR	3'-1"	8
	H4	20	#4	1	3'-3"	43
1,-0	H5	4	#4	STR	8′-9″	23
	N1	4	#4	2	8'-2"	22
	N2	4	#4	2	7'-1"	19
<u>3″ 1′-8¾″</u>	N3	4	#4	2	6'-3"	17
	N4	4	#4	2	5′-5″	14
N 2 N 3 N 2 N 2 N 2 N 3 N 3 N 3 N 3 N 3	N5	4	#4	2	4'-7"	12
	Τ1	6	# 5	STR	10'-0"	63
	V1	4	#4	STR	6'-1"	16
	V2	4	#4	STR	5'-1"	14
	٧3	4	#4	STR	4'-3"	11
6'-8 ¹ /2" 5'-7 ¹ /2" 4'-9 ¹ /2" 3'-1 ¹ /2" 3'-1 ¹ /2"	V4	4	#4	STR	3′-5″	9
	٧5	4	#4	STR	2'-7"	7
	Z1	4	#4	3	4'-9"	13
	Z2	6	#4	3	4'-3"	17
	Z3	4	#4	3	3′-9″	10
	Z4	4	#4	3	3'-4"	9
31/2	Z5	4	#4	3	2'-11"	8
		NFORC		ΤΓΓΙ		
4'-3" . 6" .		2 WI			39	6 LBS
3'-9" 6"	CLA	SS A	CONCR	ETE		
		2 WI			6.9	9 CY
3'-3"			DWALL		0.	
2'-10" 6"				IN WA		
		2 ED	GE BE		0.	
2'-5" 6"				тот	AL 8.0	0 CY
) нк. '						
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3						

I	PROJEC	CT NO.	<u>SS-</u>	4914	30
		MACON			
	STATION: <u>12+54.00</u>				-L-
	<u>SHEET 5 C</u>)F 6			
TH CAROLINA	DEPA		E OF NORTH CAR OF TRAI RALEIGH	OLINA NSPORTA	TION
OFESSION FERRING		١	VINGS FOR	5	
THE REPORT OF TH	CONC	RETE	BOX	CUL	VERT
DocuSigned by: Wael Orafat 4139C12A32AB406	H = 6'	-	O° SKE		= 2:1
6/20/2016		REVIS			SHEET NO. C-5
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SIGNATURES COMPLETED	2		4		6

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										OR RATIN RETE BO			TS			
										STRENGTH	I LIM	IT ST	ATE			
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE ELEMENT	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEME TYPE TYPE	DISTANCE FROM LEFT END OF ELEMENT (f†)	COMMENT NUMBER
		HL-93 (INVENTORY)	NZA	1	2.47		1.75	2.47	1	TOP SLAB	3.33	2.79	1	BOTTOM SLAB	0.84	
DESIGN		HL-93 (OPERATING)	N/A		3 . 21		1.35	3.21	1	TOP SLAB	3.33	3.62	1	BOTTOM SLAB	0.84	
LOAD RATING		HS-20 (INVENTORY)	36.000	2	3.75	134.90	1.75	3.75	1	TOP SLAB	3.33	3.90	1	BOTTOM SLAB	0.84	
		HS-20 (OPERATING)	36.000		4.86	174.87	1.35	4.86	1	TOP SLAB	3.33	5.05	1	BOTTOM SLAB	0.84	
		SNSH	13.500		6.78	91.53	1.40	6.78	1	TOP SLAB	3.33	7.14	1	BOTTOM SLAB	0.84	
	ш	SNGARBS2	20.000		6.40	127.96	1.40	6.40	1	TOP SLAB	3.33	6.70	1	BOTTOM SLAB	0.84	
	VEHICLI	SNAGRIS2	22.000		6.78	149.15	1.40	6.78	1	TOP SLAB	3.33	7.14	1	BOTTOM SLAB	0.84	
	VEH (V)	SNCOTTS3	27.250	3	3.09	84.27	1.40	3.09	1	TOP SLAB	3.33	3.49	1	BOTTOM SLAB	0.84	
	SLE (S	SNAGGRS4	34.925		4.06	141.73	1.40	4.06	1	TOP SLAB	3.33	4.59	1	BOTTOM SLAB	0.84	
	SINGLI	SNS5A	35 . 550		3.67	130.31	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
	•••	SNS6A	39.950		3.67	146.44	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
LEGAL LOAD		SNS7B	42.000		3.67	153.96	1.40	3 . 67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
RATING	ILER	TNAGRIT3	33.000		6.78	223.73	1.40	6.78	1	TOP SLAB	3.33	7.14	1	BOTTOM SLAB	0.84	
	TRAI	TNT4A	33.075		3.67	121.24	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
	T) The section of the	TNT6A	41.600		3.67	152.49	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
	L S L	TNT7A	42.000		3.67	153.96	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
	2 5	TNT7B	42.000		3.67	153.96	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	
	TRAC	TNAGRIT4	43.000		3.50	150.35	1.40	3.50	1	TOP SLAB	3.33	3 . 97	1	BOTTOM SLAB	0.84	
	TRUCK	TNAGT5A	45.000		3.58	161.06	1.40	3.58	1	TOP SLAB	3.33	4.02	1	BOTTOM SLAB	0.84	
		TNAGT5B	45.000		3.67	164.95	1.40	3.67	1	TOP SLAB	3.33	4.12	1	BOTTOM SLAB	0.84	



LRFR SUMMARY

(LOOKING DOWNSTREAM)

ASSEMBLED BY : H CHECKED BY : V	. T. BARBO		-	4-28-16 5-9-16
DRAWN BY : WMC CHECKED BY : GM	7/ 7/	REV. 10/1/11		MAA/GM

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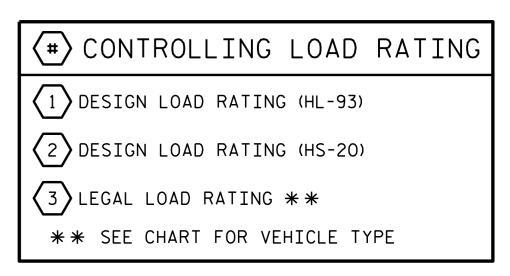
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LOAD FACTORS:

DESIGN LOAD	RAIING	FACTORS
LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

DESTON LOAD RATING FACTORS

NOTE: RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.



	PROJECT NO. <u>SS-4914B0</u> <u>MACON</u> COUNTY STATION: <u>12+54.00</u> -L- SHEET 6 OF 6	
Pocusigned by: Wall Qrafat 4199012A32AB406	DEPARTMENT OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)	
6/20/2016	REVISIONS SHEET N	0.
OCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO.BY:DATE:NO.BY:DATE:C-6133TOTAL SHEETS246	
	STD. NO. LRFR5	

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRESS IN EXTREME TIDER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN	
OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE

AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH JANUARY, 1990

STD. NO. SN